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Design Storms and Parameters for Sustainable Drainage Systems



Gilles Rivard, ing. M. Sc.
Vice-President – Urban Hydrology (Lasalle-NHC)

SETTING THE CONTEXT

The world is my representation

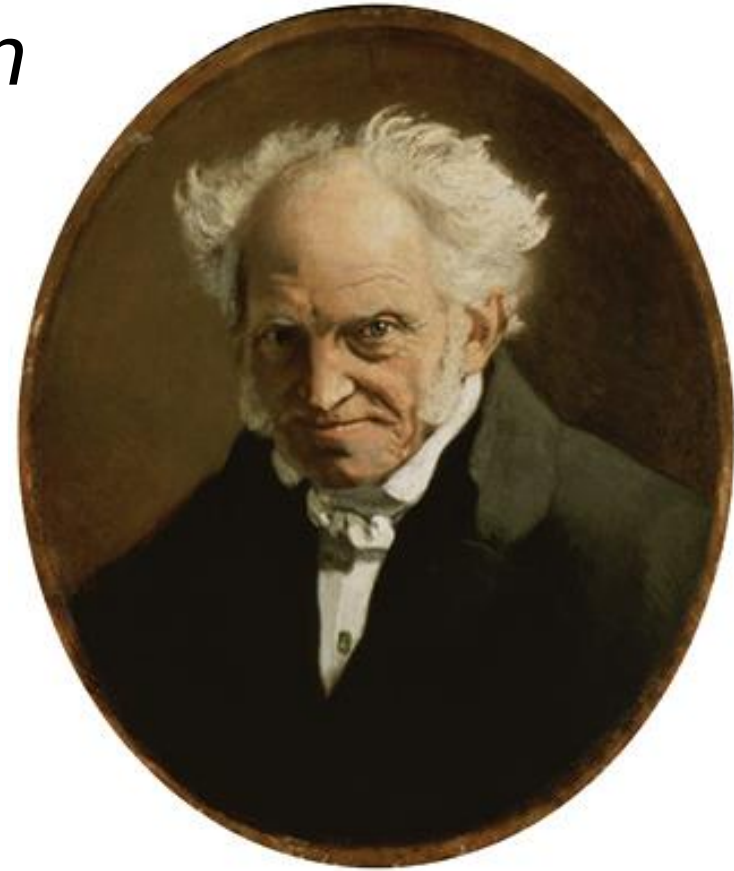
(The World as Will and Representation -1819)

NOUMENA

*Facts as they are and
in themselves – Reality*

PHENOMENA

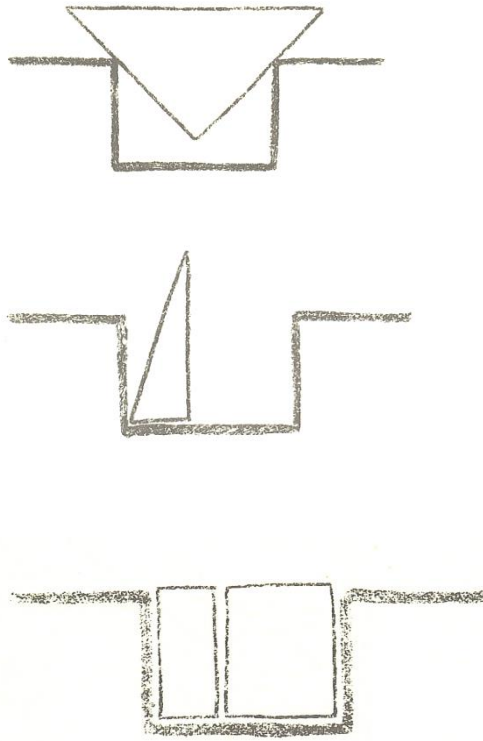
*What can be known
(our models)*



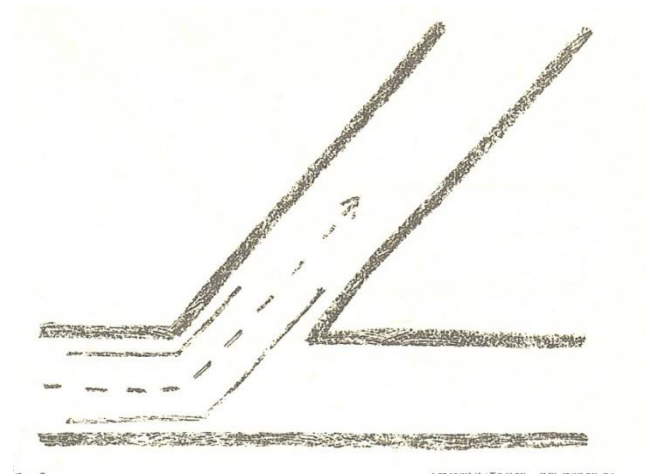
SCHOPENHAUER

SETTING THE CONTEXT

Optimal Solutions

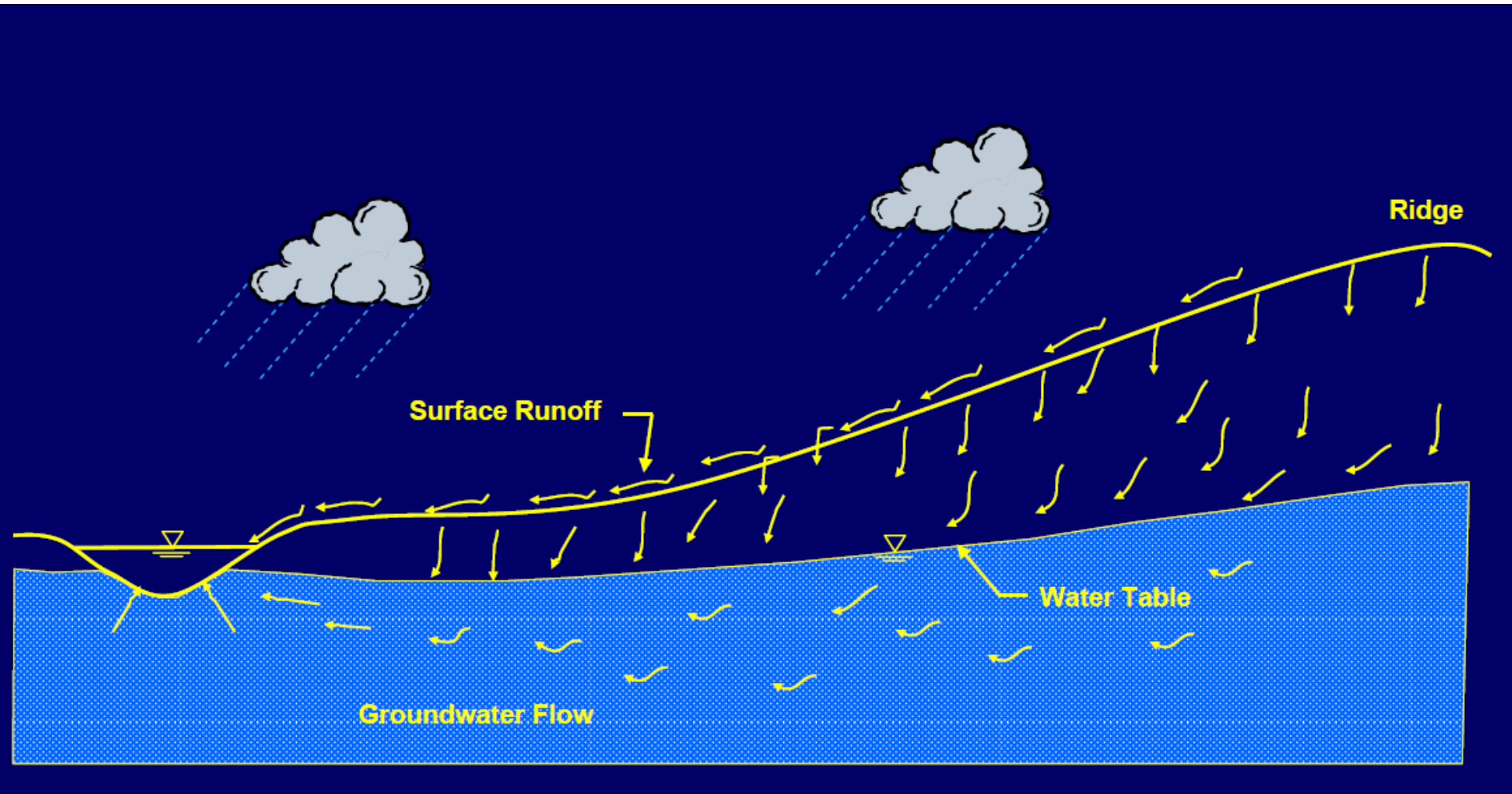


New paradigm



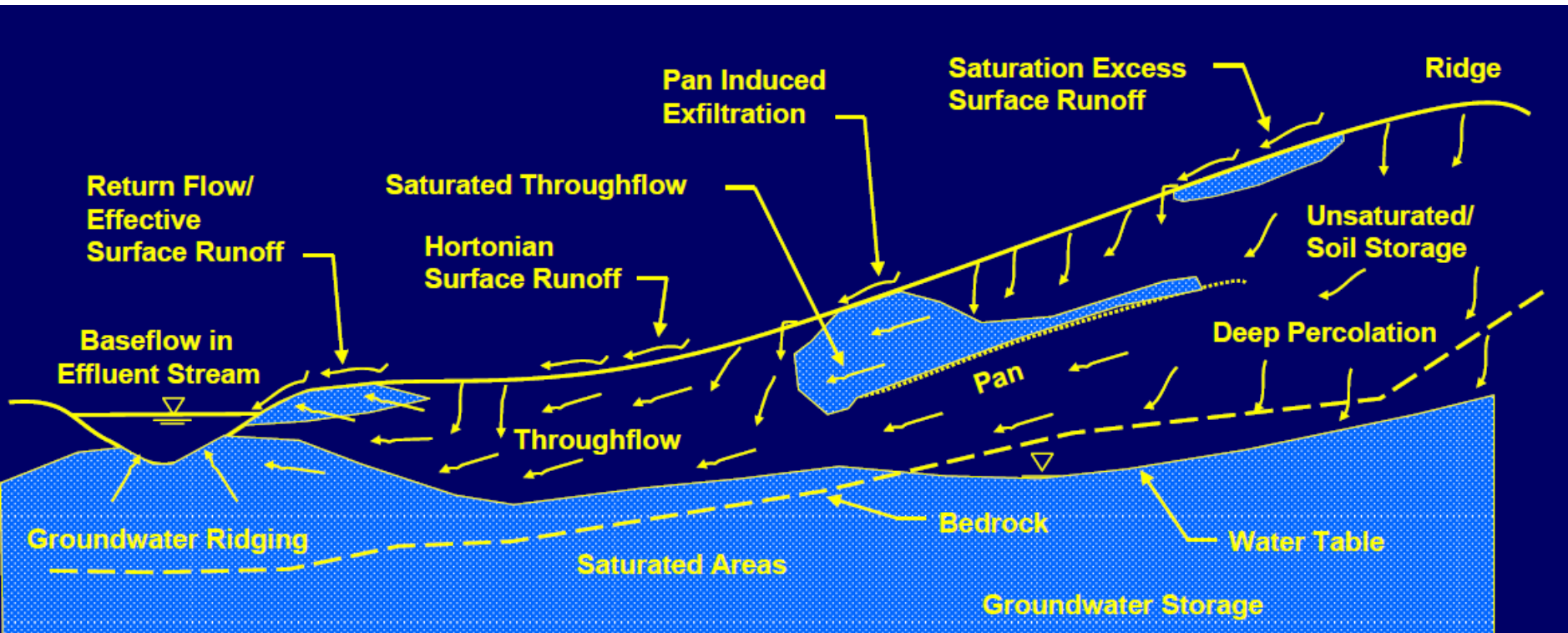
Recreate or maintain the natural processes for drainage instead of rapid and efficient runoff

SETTING THE CONTEXT



Usual Modeling – Hortonian flow

SETTING THE CONTEXT



Partial Components of Hillslope Hydrology

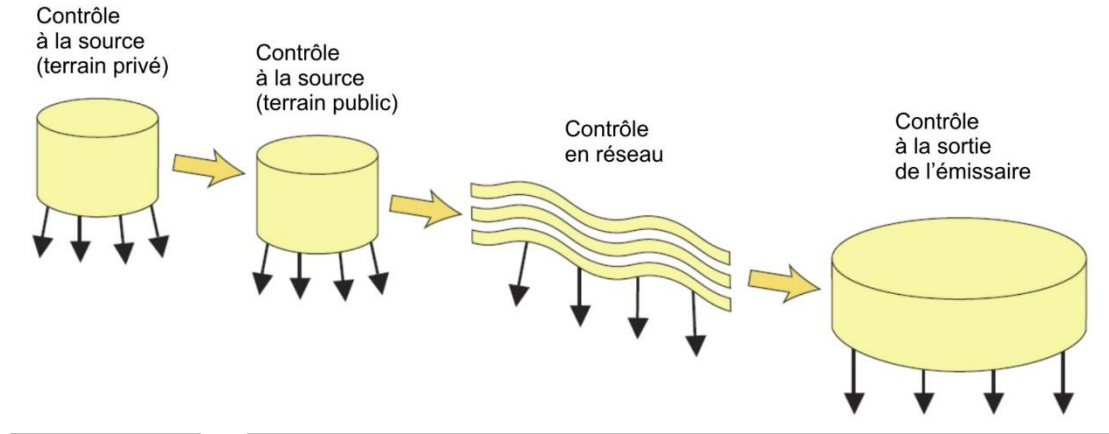
Adapted from Chorley (1978)

Reality much more complex –
different types of flow

SETTING THE CONTEXT

NEW PARADIGM

- Treatment train



Terrain privé

Terrain public

- Complete rainfall spectrum

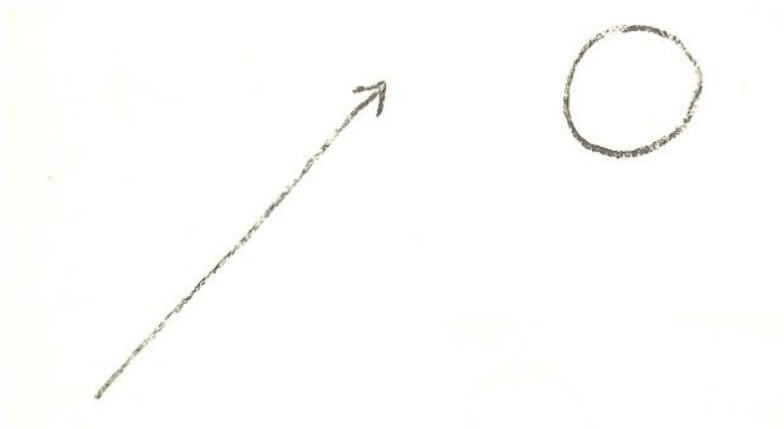
- Runoff volume reduction



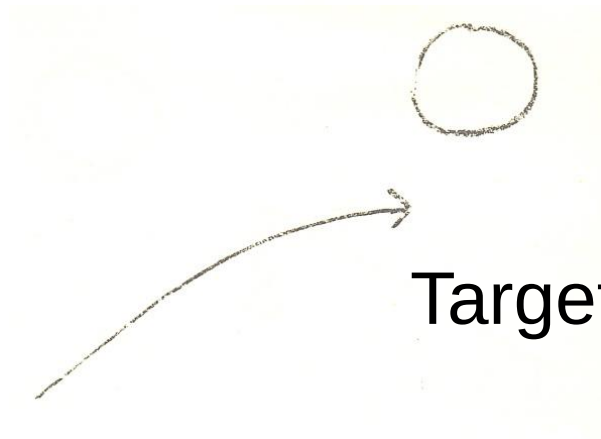
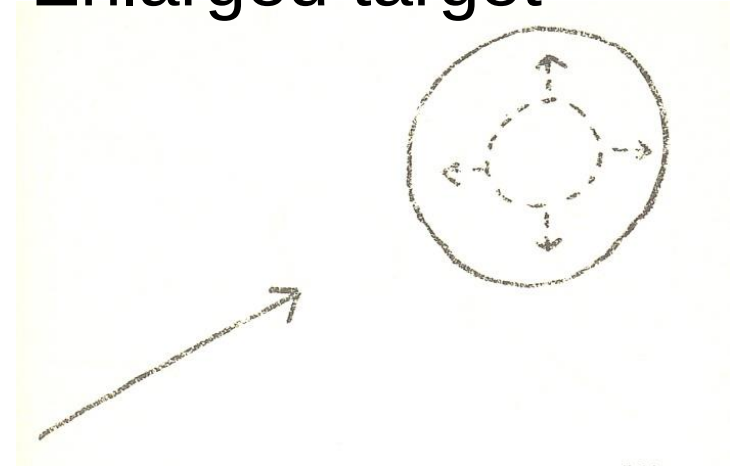
SETTING THE CONTEXT

DEFINING OBJECTIVES

Wrong target



Enlarged target

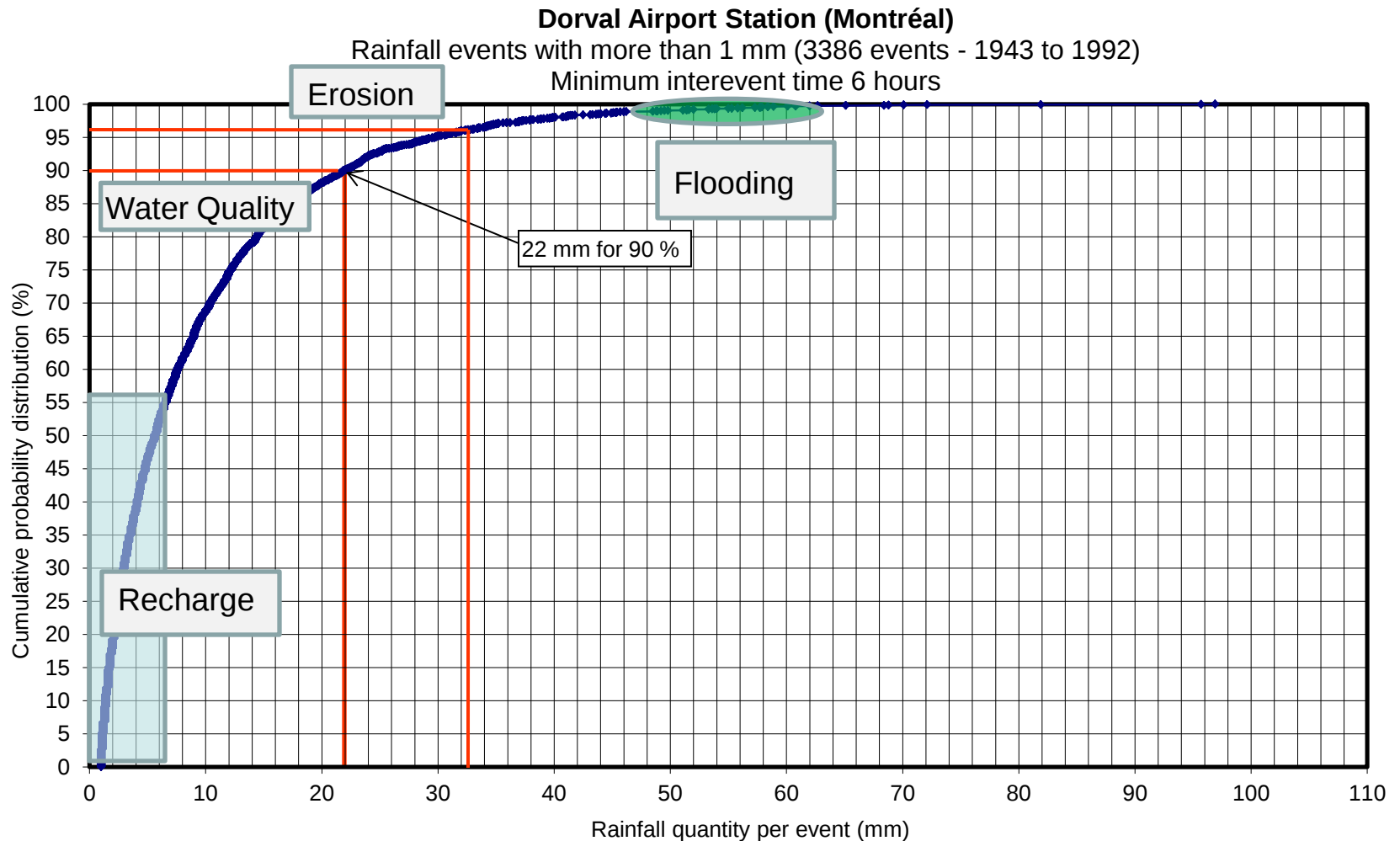


Target not reached

OUTLINE

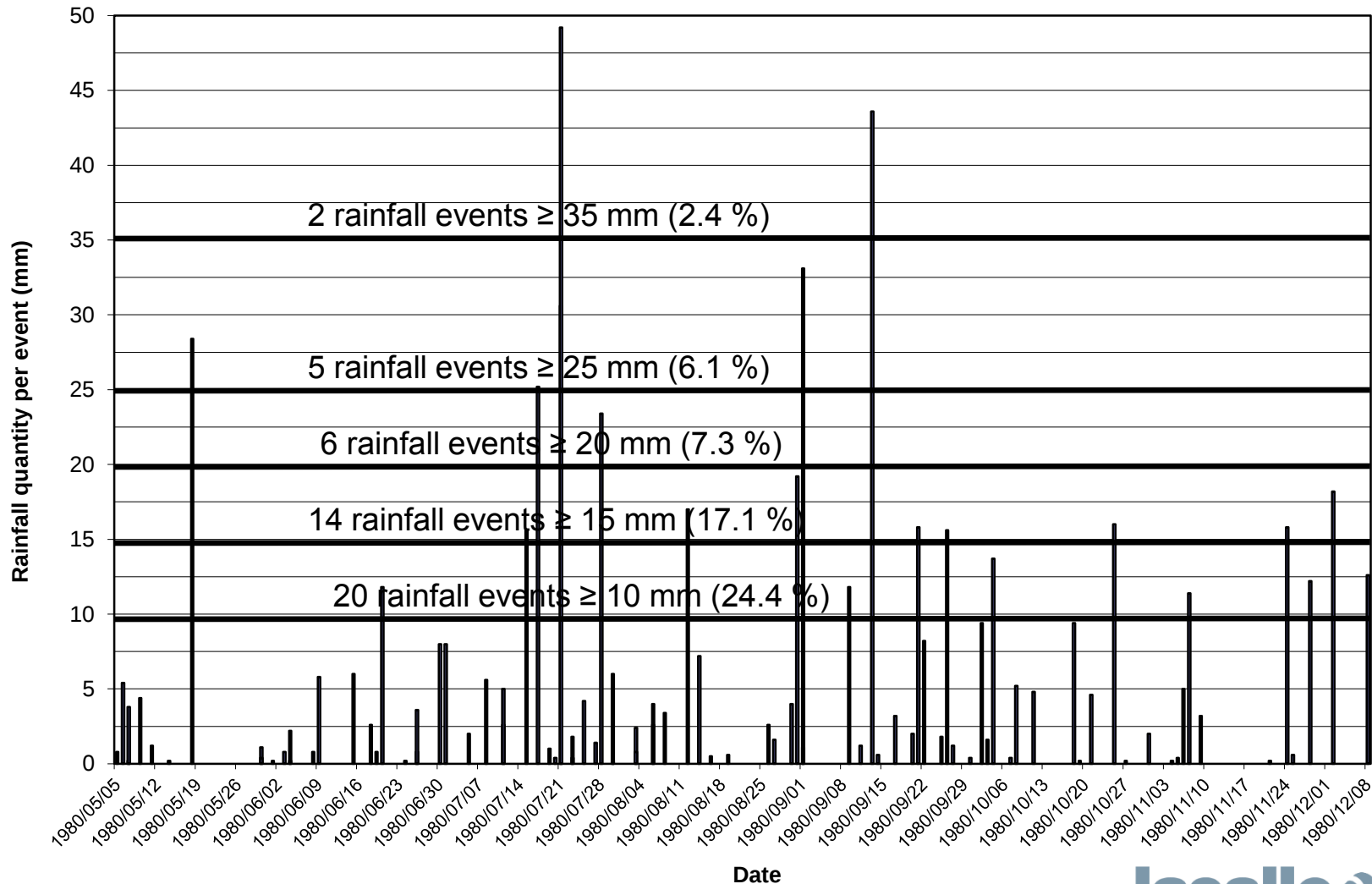
- ❑ Urban Runoff Quality Control in Context
- ❑ Approaches and Parameters for Design Storms
- ❑ Designing and Validating Design Storms with Continuous Simulations
- ❑ Conclusion

URBAN RUNOFF QUALITY CONTROL



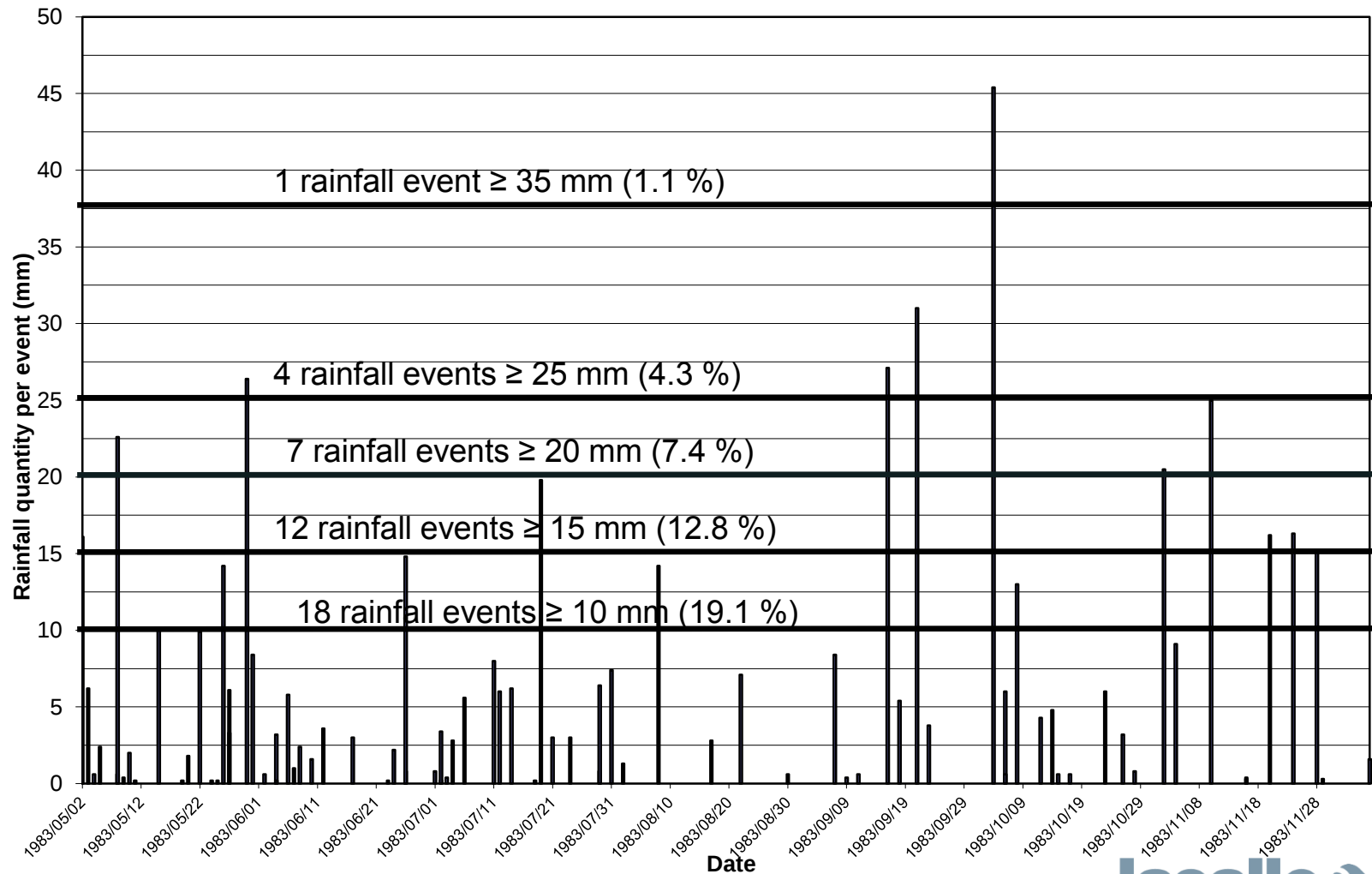
URBAN RUNOFF QUALITY CONTROL

1980 - 82 rainfall events

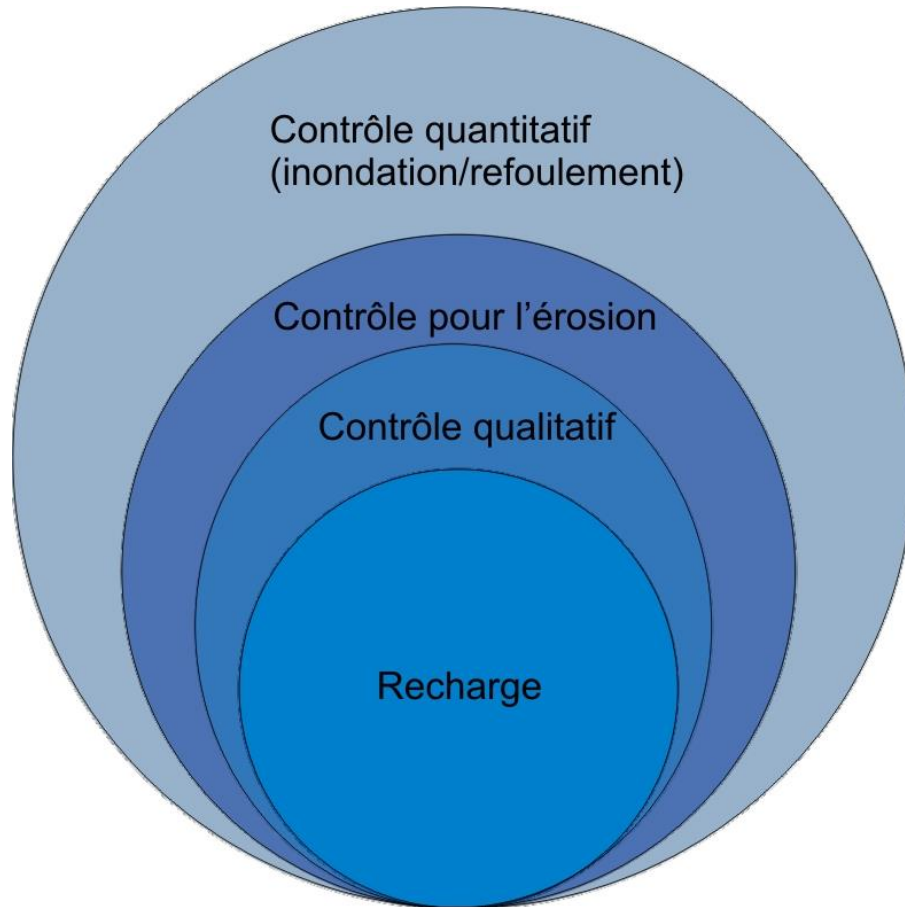


URBAN RUNOFF QUALITY CONTROL

1983 - 94 rainfall events



URBAN RUNOFF QUALITY CONTROL



- Recharge – 5 à 10 mm
- **Quality** – 90 % of annual runoff
- Channel protection – 1:1 y with 24 to 48 hours
- Quantity – 1:2 y, 1:10 y and 1:100 y as for pre-development conditions

APPROACHES AND PARAMETERS

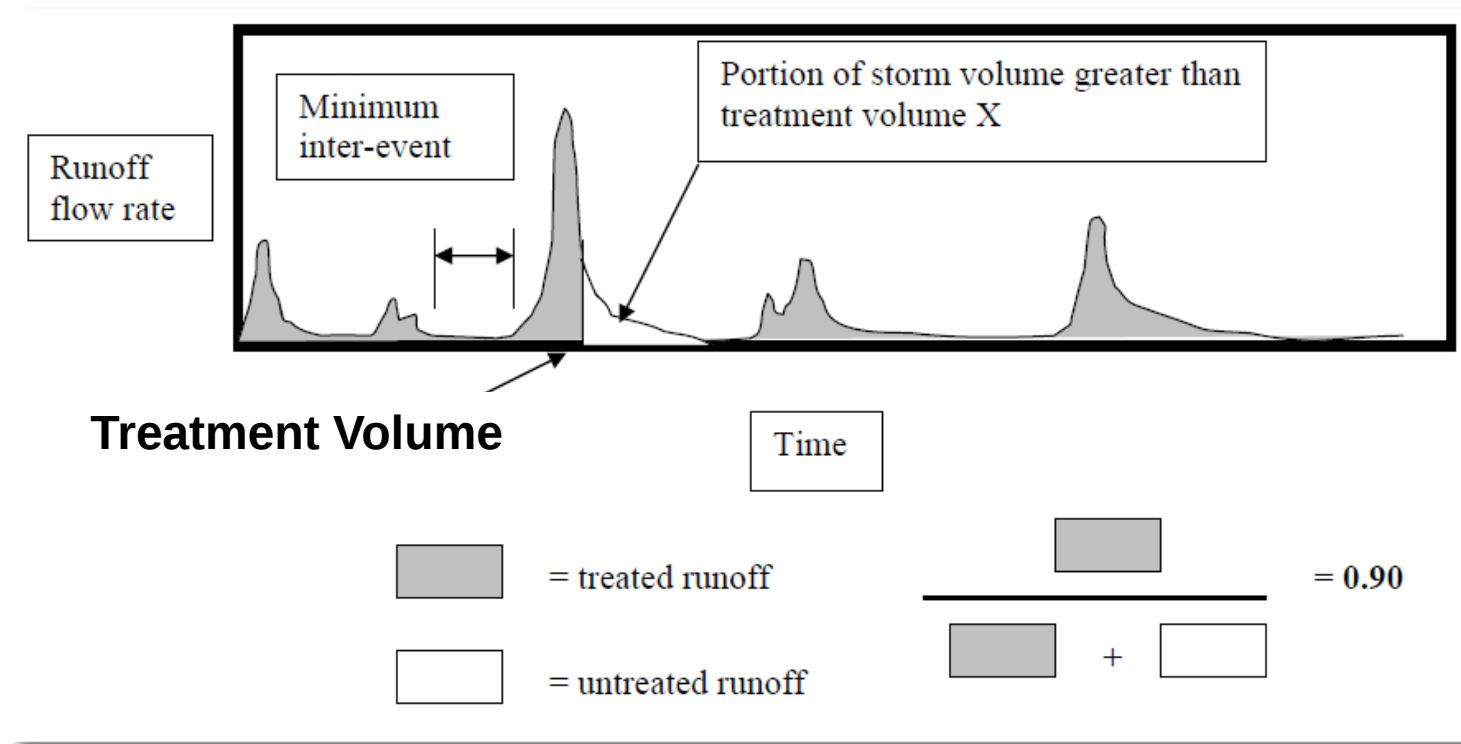
DRAINAGE NETWORK COMPONENTS ARE
DESIGNED WITH DIFFERENT PARAMETERS

- **Volume** (retention basin, bioretention)
- **Discharge** (pipes, swale, hydrodynamic devices)



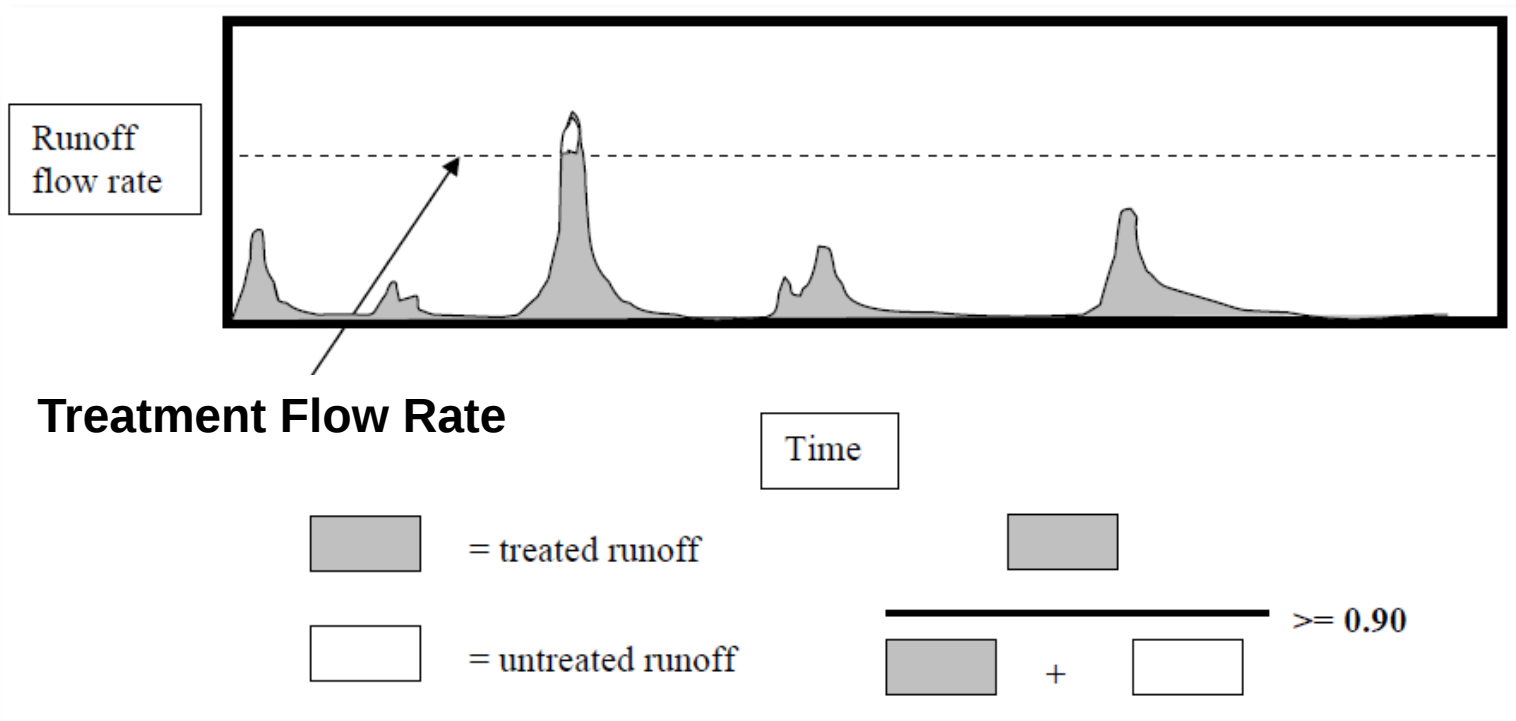
APPROACHES AND PARAMETERS

Volume based treatment systems



APPROACHES AND PARAMETERS

Rate based treatment systems



APPROACHES AND PARAMETERS

Volume based treatment systems

$$\text{WQ Volume} = R_v \times P \times A$$

Where R_v = Volumetric runoff coefficient

P = Precipitation for 90 % of the storms

A = Tributary area

- Basic assumption – runoff volume (and not peak discharge) is the main parameter for water quality control
- **Application in design:** retention basin; bioretention

APPROACHES AND PARAMETERS

Rate based treatment systems

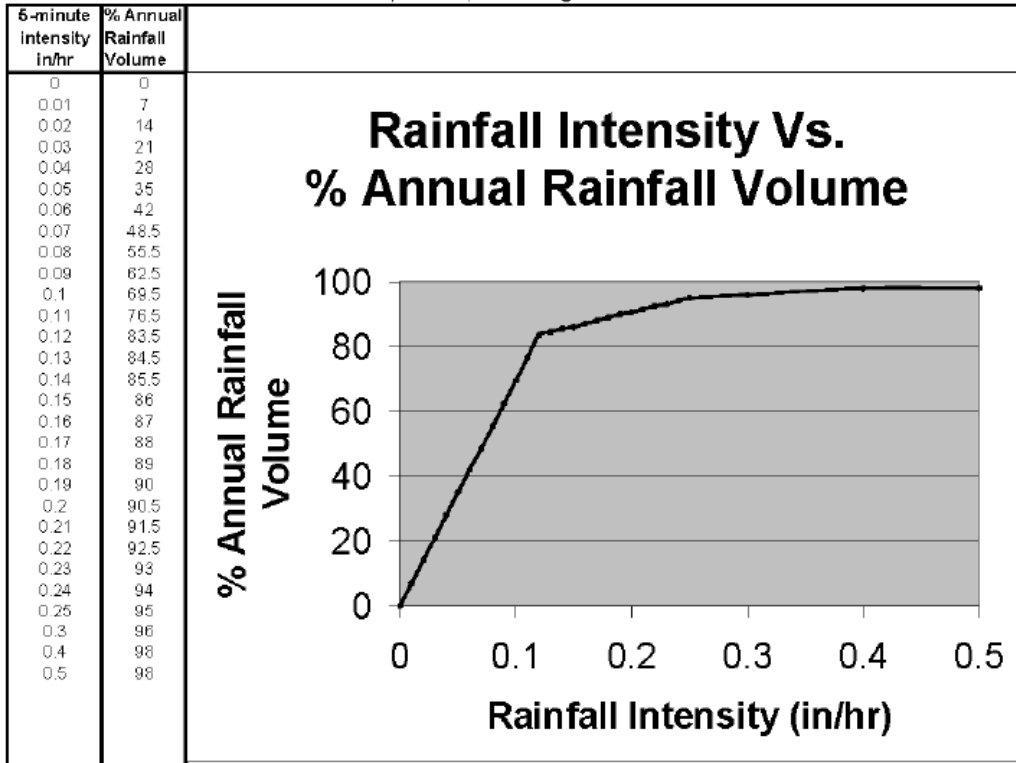
Approach	Advantage	Disadvantage
Uniform rainfall intensity	Simplest method	Doesn't take into account rainfall patterns or statistics
Rational method	Simple method	Need specific IDF curves (high occurrences)
Modelling with design storms	Intermediate complexity	Need to determine design storms (could be different for peak Q or Volumes)
Modelling with continuous simulation	Optimal approach, both for volume and rate based treatment systems	Data might not be available

APPROACHES AND PARAMETERS

Example with uniform intensity

Figure 4: Rainfall Intensity Versus Percentage of Annual Rainfall Volume
Rainfall data taken from each of four Portland quadrants, then averaged.

8/12/2004
Fancher



90 % : 4.8 mm/hr
(too low ??)

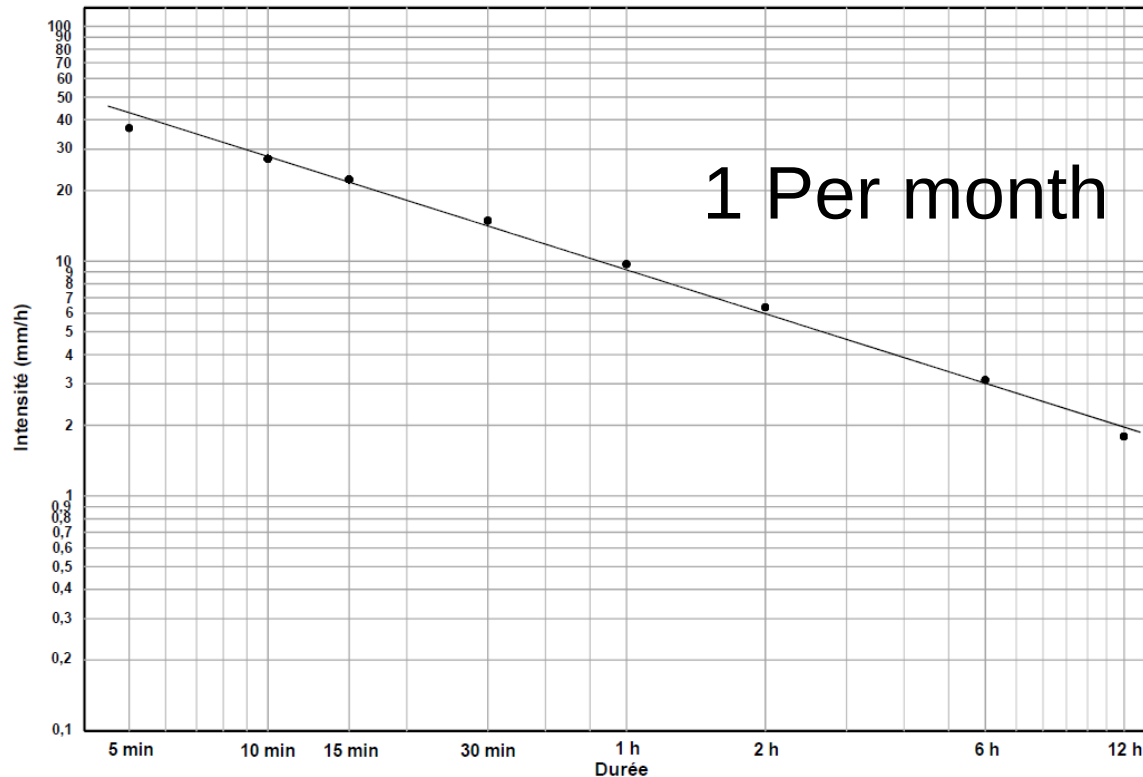
Portland (Oregon) SWM Manual

APPROACHES AND PARAMETERS

Use with rational method ($Q = CIA/360$) High occurrence IDF curves

Développement durable,
Environnement et Lutte
contre les changements
climatiques
Québec

INTENSITÉ, DURÉE, FRÉQUENCE (IDF) DES PLUIES DE GRANDES RÉCURRENCES



Station : Saint-Jérôme
7037400

Latitude : 45° 48' 23" Altitude : 169 m
Longitude : 74° 02' 22"

Période disponible : 1981-2010

Récurrance demandée : 1 fois par mois

Tableau des valeurs

Durée	Réc. max.	Int. (mm/h)	ln(Int.)	
			Valeur	Erreur type
5 min	8,75	36,86	3,61	0,03
10 min	9,74	27,28	3,31	0,01
15 min	9,97	22,27	3,10	0,02
30 min	10,34	14,86	2,70	0,02
1 h	10,53	9,70	2,27	0,02
2 h	10,65	6,36	1,85	0,02
6 h	10,77	3,11	1,14	0,03
12 h	10,79	1,79	0,58	0,02

Tableau des paramètres de Montana

Paramètre	Valeur	Écart-type
ln(a)	4,76	0,10
b	0,620	0,018

Équation de la courbe IDF

$$\text{Intensité (mm/h)} = a \cdot [\text{Durée (min)}]^{-b}$$

APPROACHES AND PARAMETERS

Selected approaches (Quebec Provincial Guidelines)

- **Small Systems**
 - Volume based $WQ \text{ Volume} = R_v \times P \times A$
 - Rate Based Rational method
IDF Curves (65 % of 2 yr IDF)
- **Larger Systems**
 - Design Storms both for Volume and Rate based SCM

Both approaches should give similar results !!...

DESIGNING DESIGN STORMS

An old problem...

RELIABILITY OF DESIGN STORMS IN MODELLING

BY

BEN URBONAS

CHIEF, MASTER PLANNING PROGRAM

URBAN DRAINAGE AND FLOOD CONTROL DISTRICT

DENVER, COLORADO

SEMINAR ON THE DESIGN STORM CONCEPT

ECOLE POLYTECHNIQUE, UNIVERSITY OF MONTREAL

MAY 23, 1979^a

1979

DESIGNING DESIGN STORMS

An old problem...

**Design Storm Events for Urban Drainage
Based on Historical Rainfall Data: A Conceptual
Framework for a Logical Approach**

Gilles Rivard

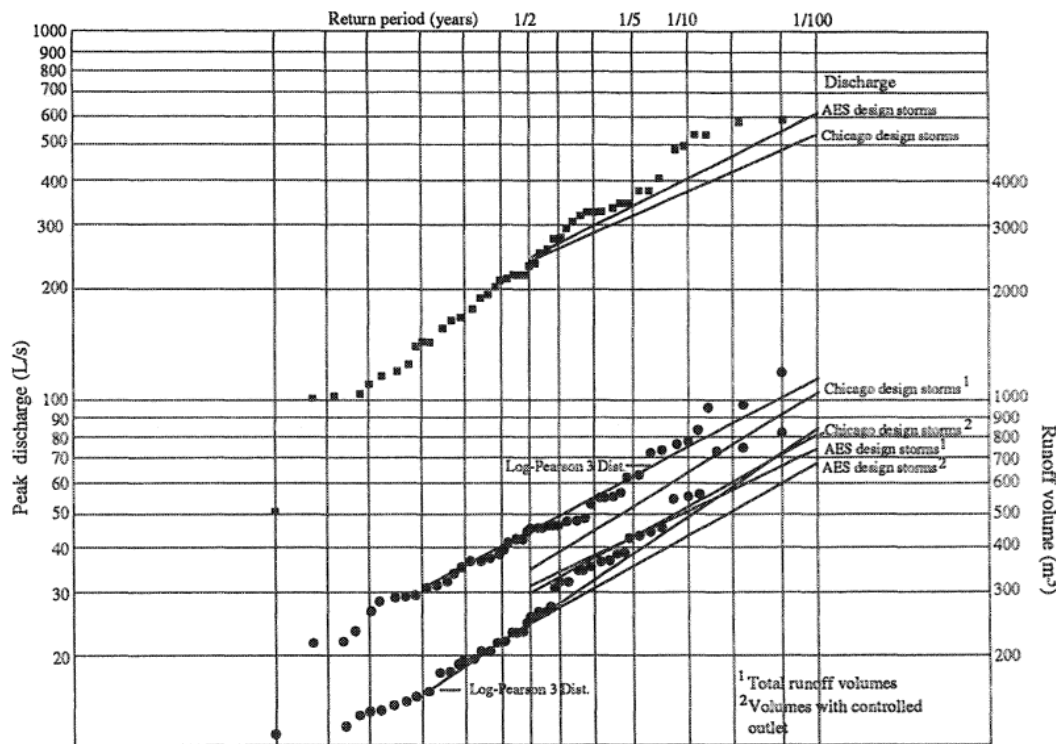
...1996

DESIGNING DESIGN STORMS

An old problem...

Design Storm Events for Urban Drainage
Based on Historical Rainfall Data: A Conceptual
Framework for a Logical Approach

...1996



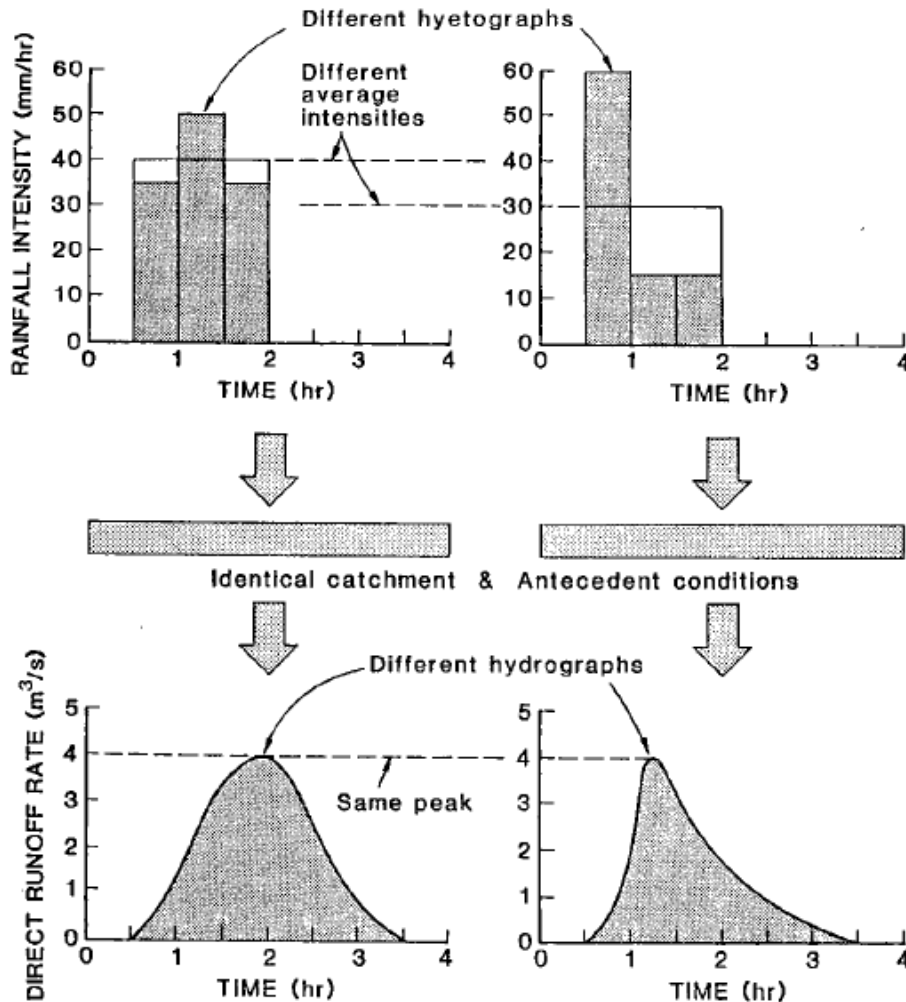
Peak discharge

Runoff Volume

Figure 12.5 Statistical analysis for commercial or industrial lot (1.5 ha 100% impervious).

DESIGNING DESIGN STORMS

Many conceptual difficulties...



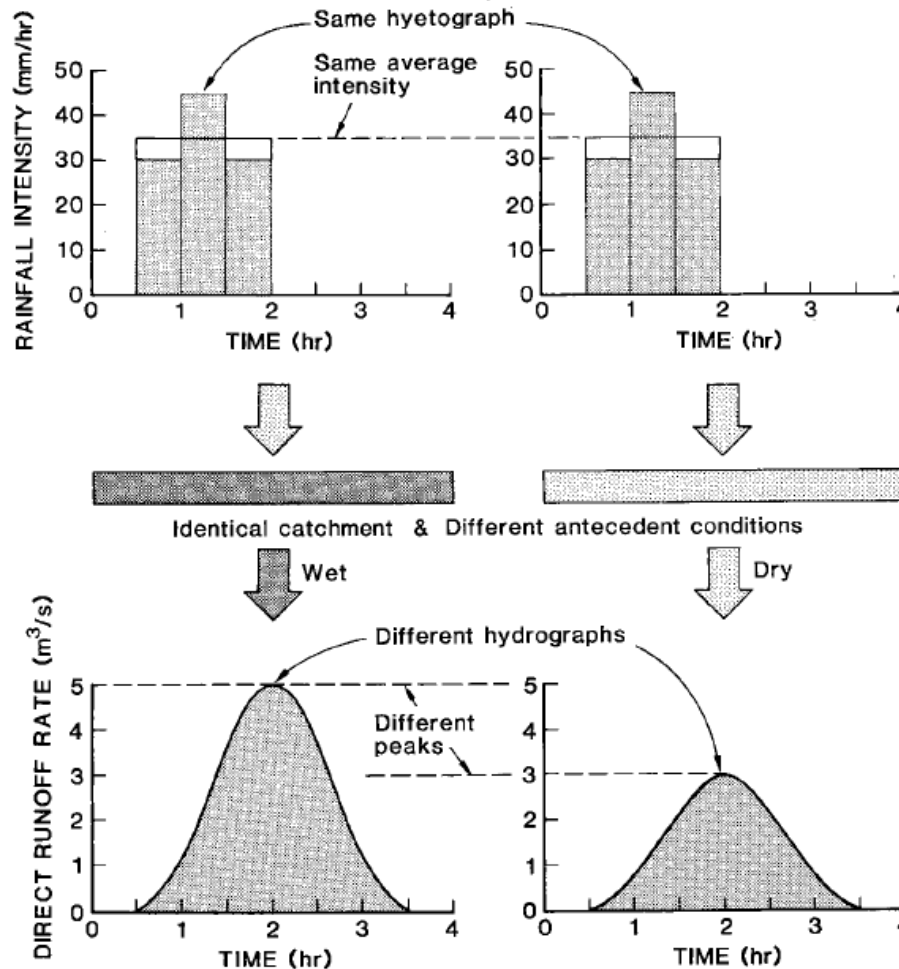
Design Storm Pathology

Barry J. Adams¹ and Charles D.D. Howard²

Different
rainfall with
same
catchment
conditions

DESIGNING DESIGN STORMS

Many conceptual difficulties...



Design Storm Pathology

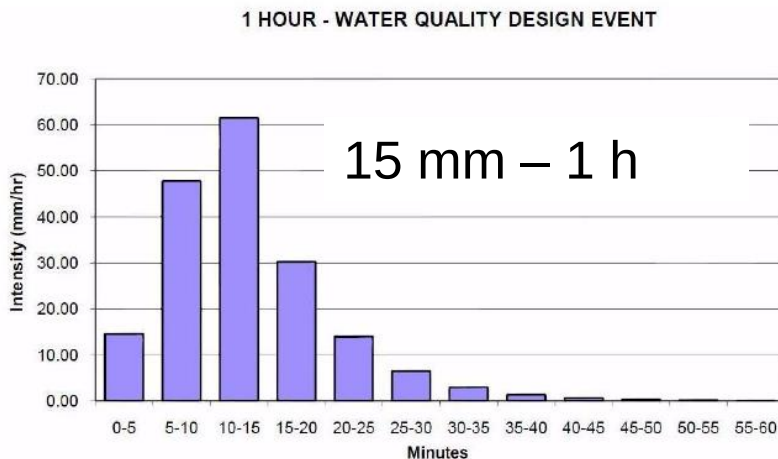
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Same
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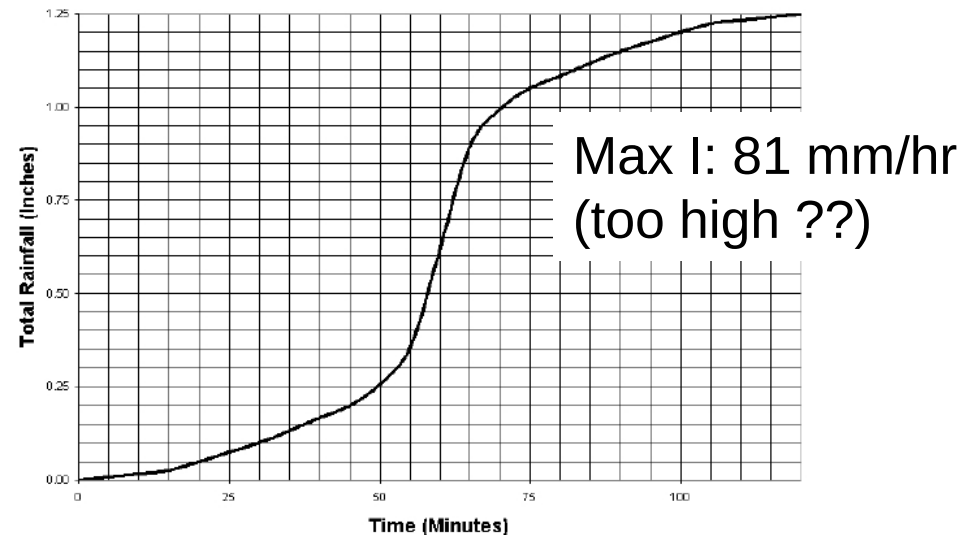
DESIGNING DESIGN STORMS

Suggested design storms for water quality

- Calgary, Canada (2011)



- New Jersey, USA (2004)



- Ontario, Canada (2003)

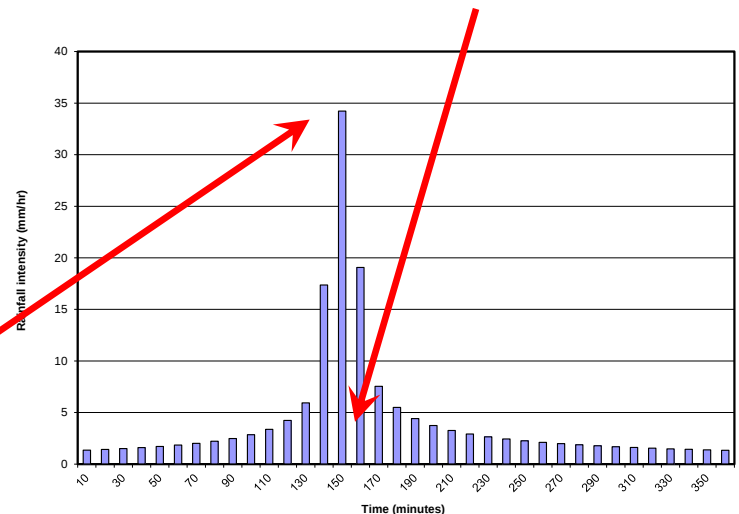
25 mm 4-hr Chicago-type storm (ponds)

25 mm 2-hr Chicago-type storm (swales)

DESIGNING DESIGN STORMS

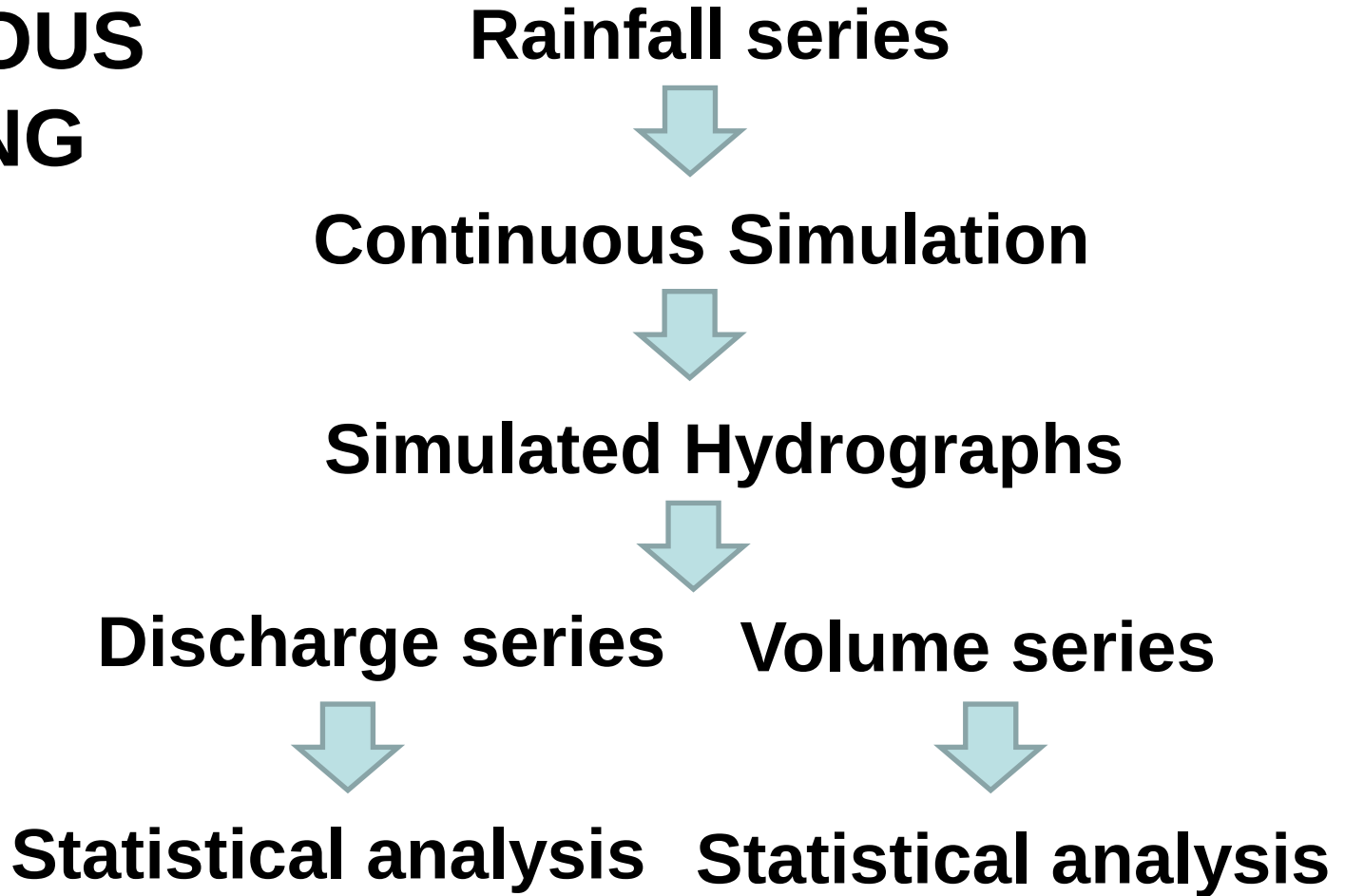
QUEBEC APPROACH

- Total rainfall quantity: 22 to 26 mm for different regions (90 % of annual rainfall)
- Chicago-type Design Storm for time distribution
 - Duration: 6 hrs
 - Ratio_{TimeToPeak} $r = 0.45$
 - Peak intensities to be adjusted



DESIGNING DESIGN STORMS

CONTINUOUS MODELLING



DESIGN STORMS VALIDATION

DESIGNING DESIGN STORMS

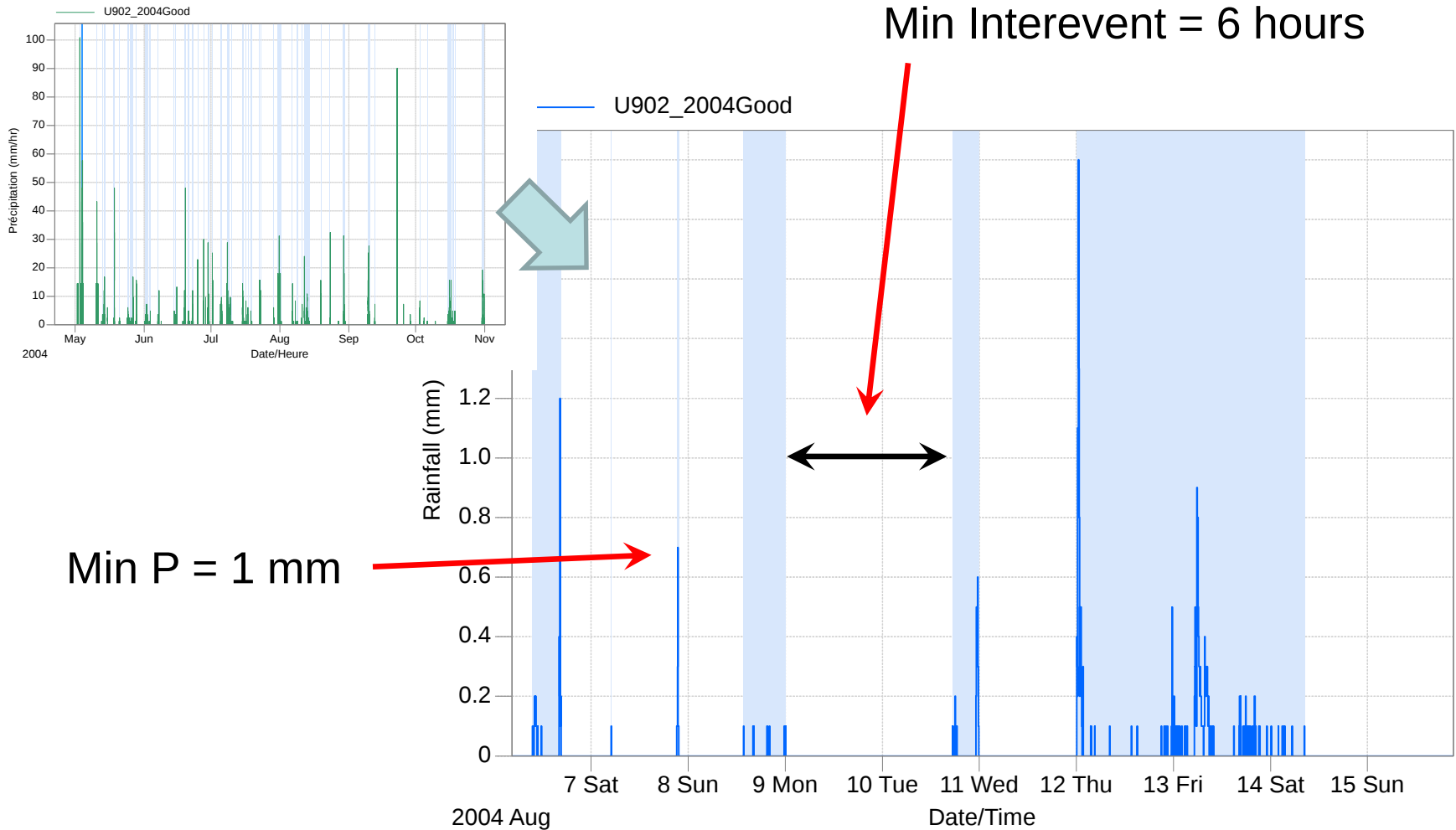
CONTINUOUS MODELLING

IDF Analysis for actual storms with less than 25 mm (and higher than 10 mm)

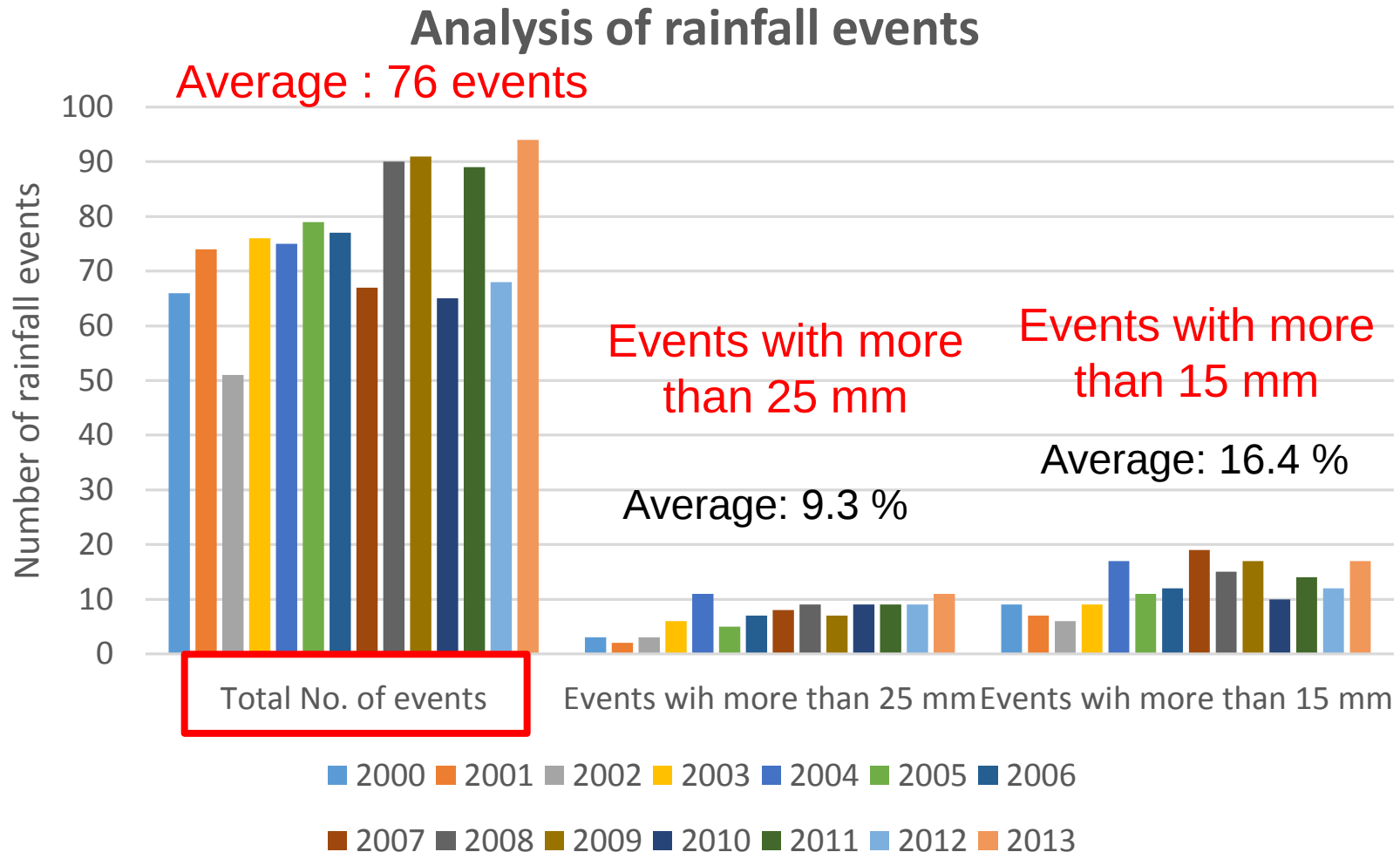
Event	Date	Duration (h)	Maximum Rainfall (mm/hr)	Mean Rainfall (mm/hr)	Total Rainfall (mm)
2	2001-05-12 13:10	11.75	4.8	0.9702	11.4
7	2001-06-02 4:45	10.58	8.4	1.606	17
10	2001-06-04 4:10	22.5	22.8	0.88	19.8
17	2001-06-22 8:25	5	25.2	2.96	14.8
18	2001-06-22 20:10	28.67	10.8	0.8198	23.5
47	2001-08-20 4:35	5.33	13.2	2.119	11.3
48	2001-08-20 23:15	2.83	50.4	4.588	13
50	2001-08-26 17:15	13.75	14.4	0.9164	12.6
60	2001-09-20 13:10	26.33	10.8	0.5203	13.7
72	2001-10-17 3:30	29.25	7.2	0.3521	10.3
75	2001-10-23 15:30	11	9.6	1.345	14.8

DESIGNING DESIGN STORMS

EXAMPLE - 2004



DESIGNING DESIGN STORMS



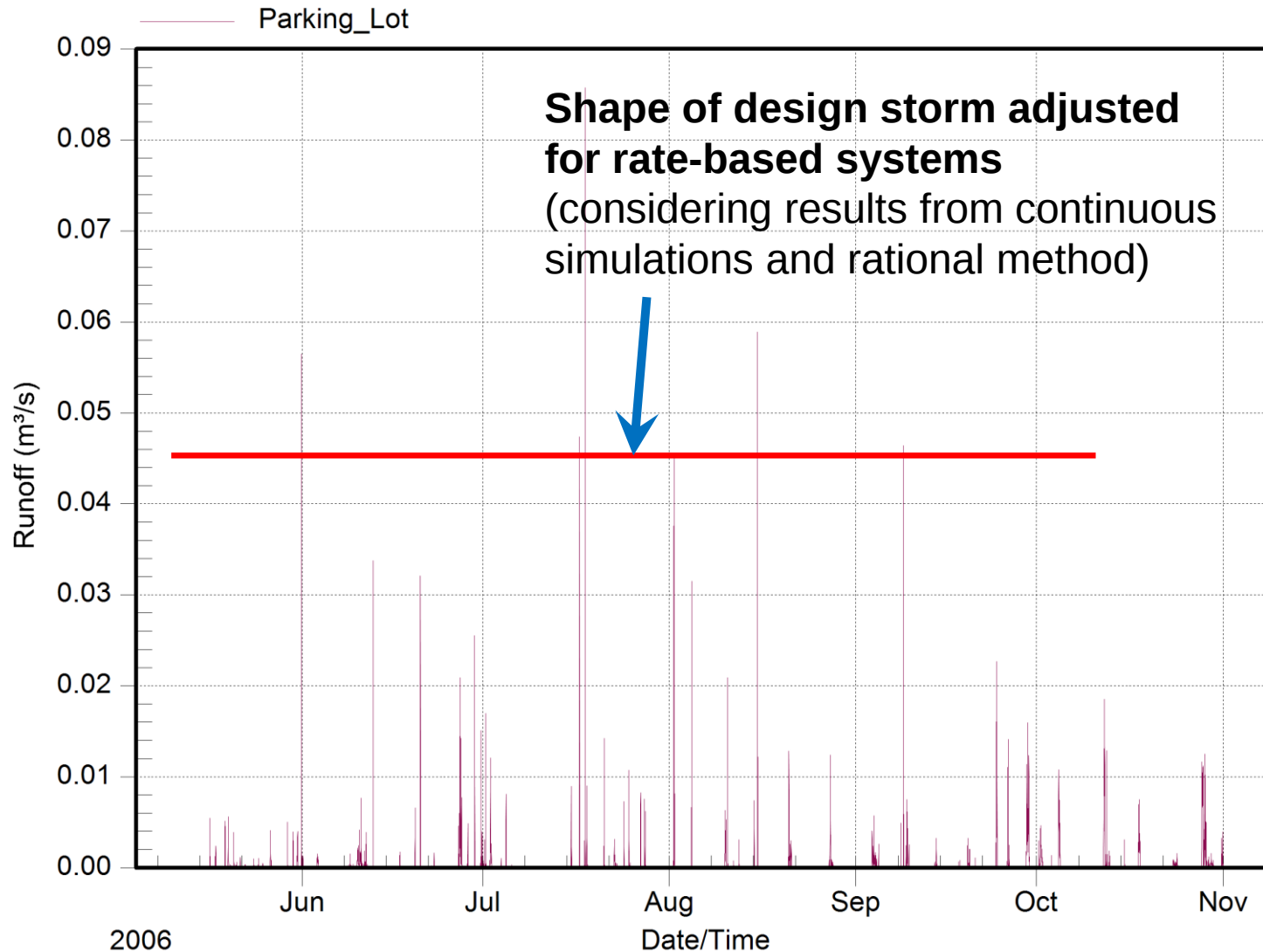
DESIGNING DESIGN STORMS

Generic types of land use defined

- Small Parking Lot
- Medium-size high density residential
- Medium-size low density residential

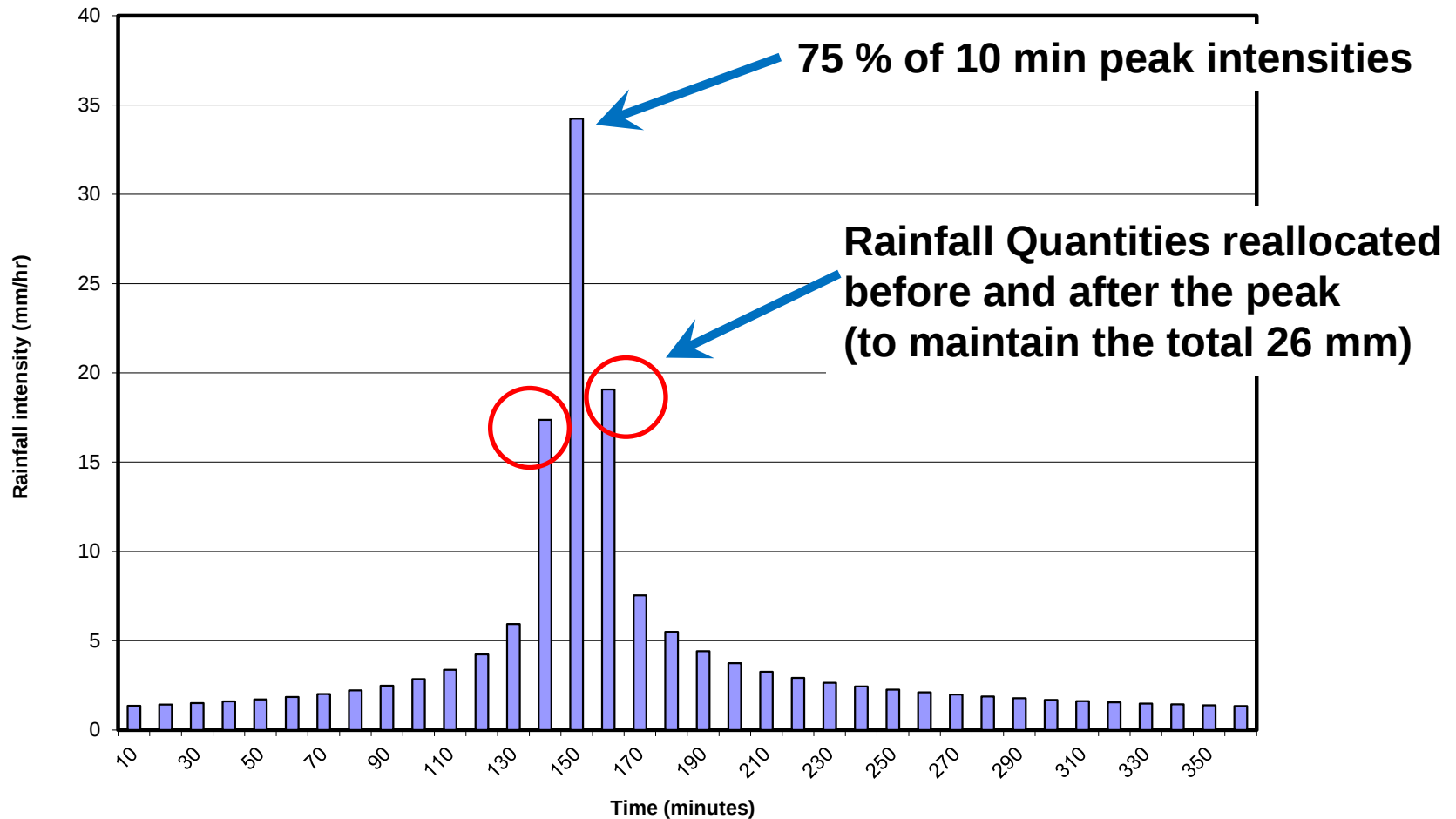
Continuous modelling used to adjust the shape of the design storm

DESIGNING DESIGN STORMS



DESIGNING DESIGN STORMS

Design storm adjustments for rate-based systems



SUMMARY – THE WAY FORWARD

- Urban Runoff Quality Control



Small Storms

- Design Tools adapted to types of SCM (Volume or Rate Based)
- Design Tools adapted to scale and complexity of project
- Ideally use actual rainfall series where available

Design Storms and Parameters for Sustainable Drainage Systems



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